

SR 1033-008 over I-83 and Mill Creek

Sherman Street Bridge Modification/Extension



**ASHE Altoona / PennDOT District 9-0
Joint Workshop**
April 23, 2026

Presented By:
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Stantec Consulting Services Inc.



Overall I-83 North York Widening Project





North York Project Corridor

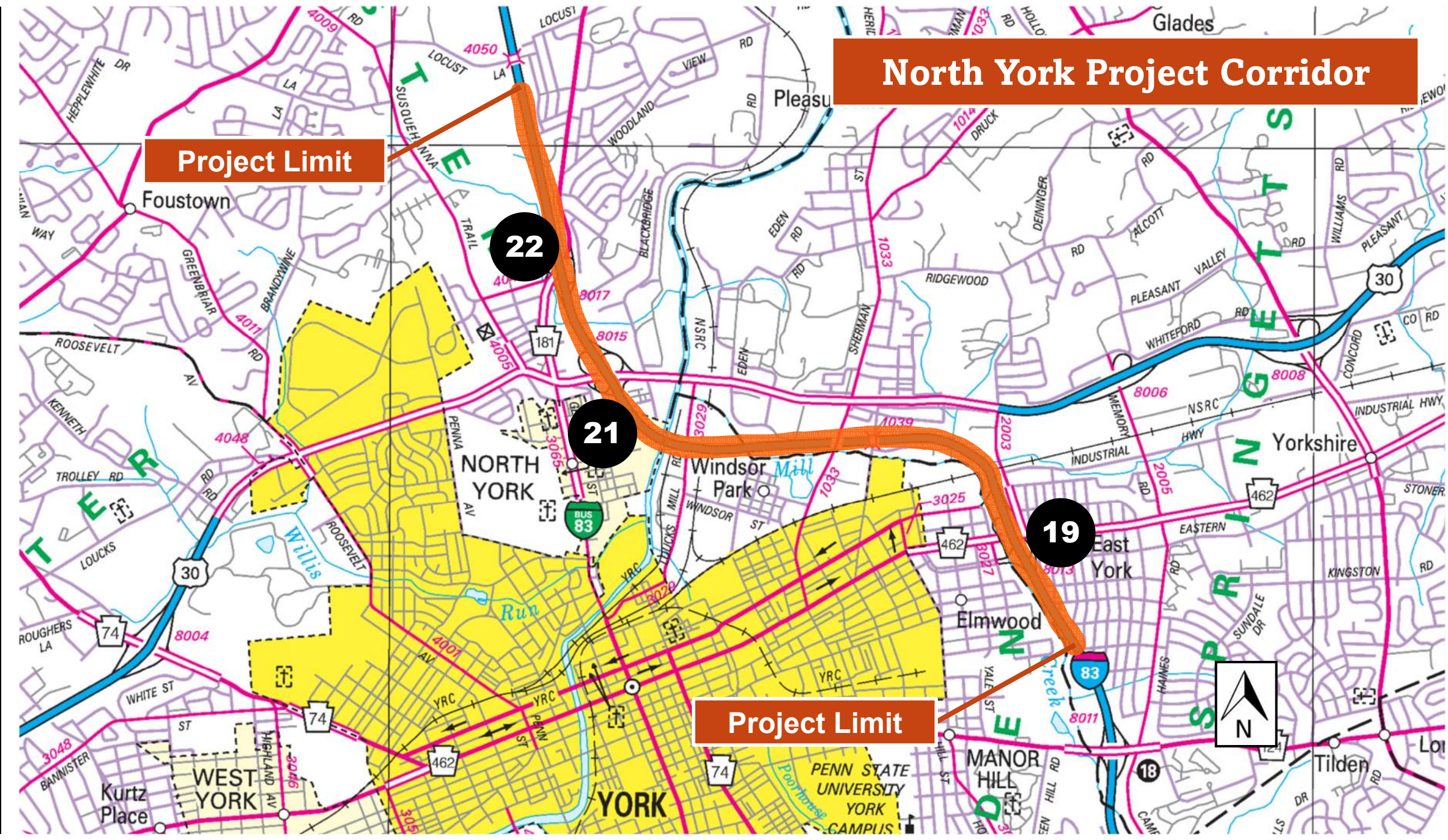
Project Limit

22

21

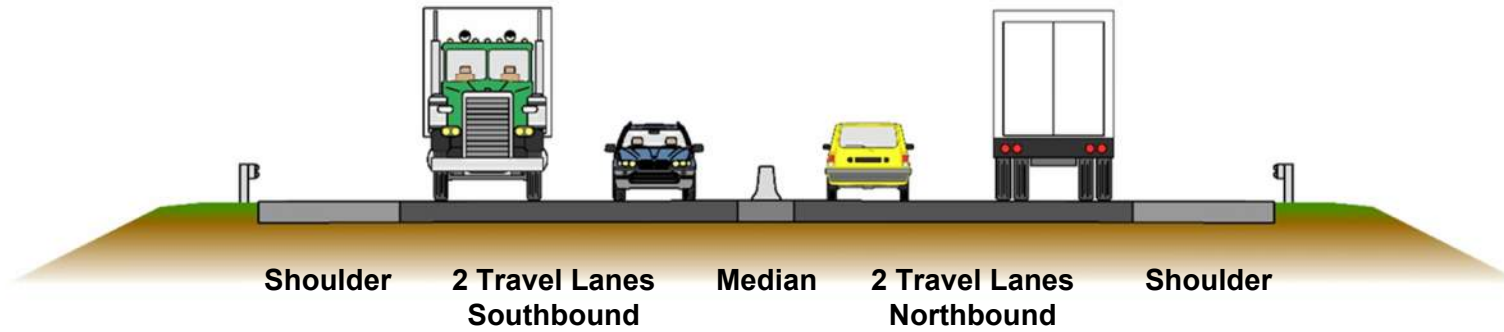
19

Project Limit

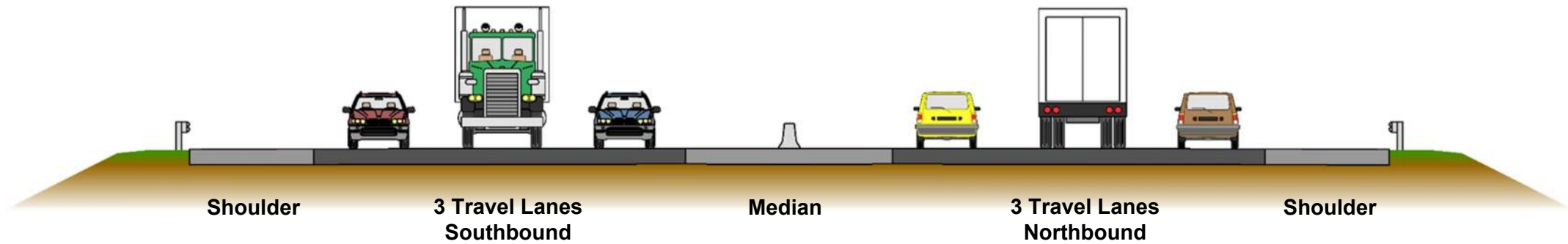




I-83: Widening to 6 Lanes



Out to Out Width of 68-ft



Out to Out Width of 122-ft

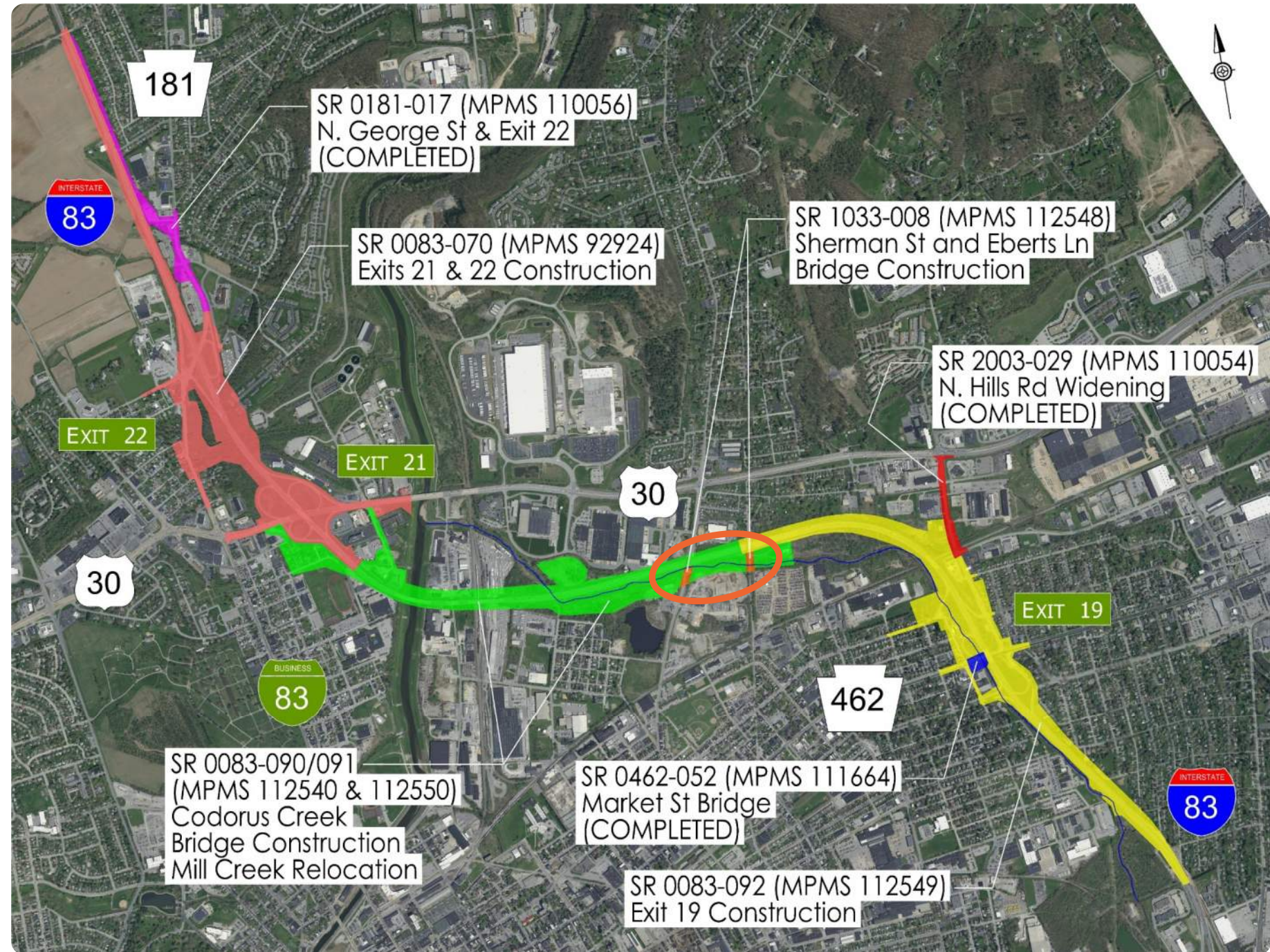




Project Delivery

3 Mainline Projects

4 Early Action Projects





Eberts Lane Bridge

Replaced the existing four-span concrete bridge with a two-span, bulb-tee beam bridge





Sherman St Bridge

Existing two-span steel plate girder bridge





Why Modify the Sherman Street Bridge?

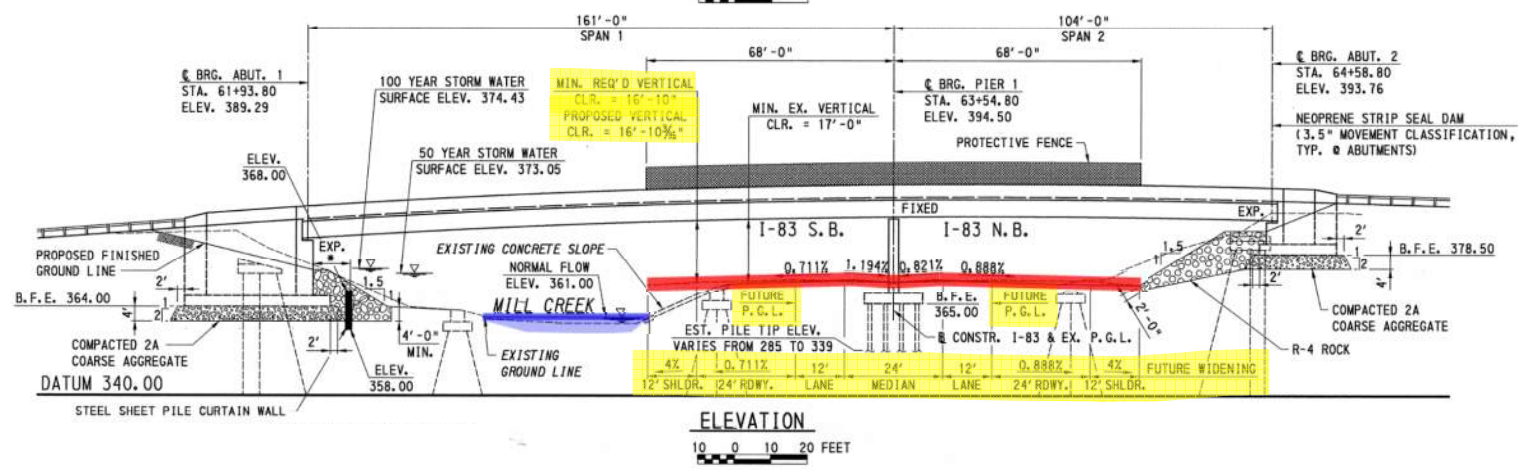
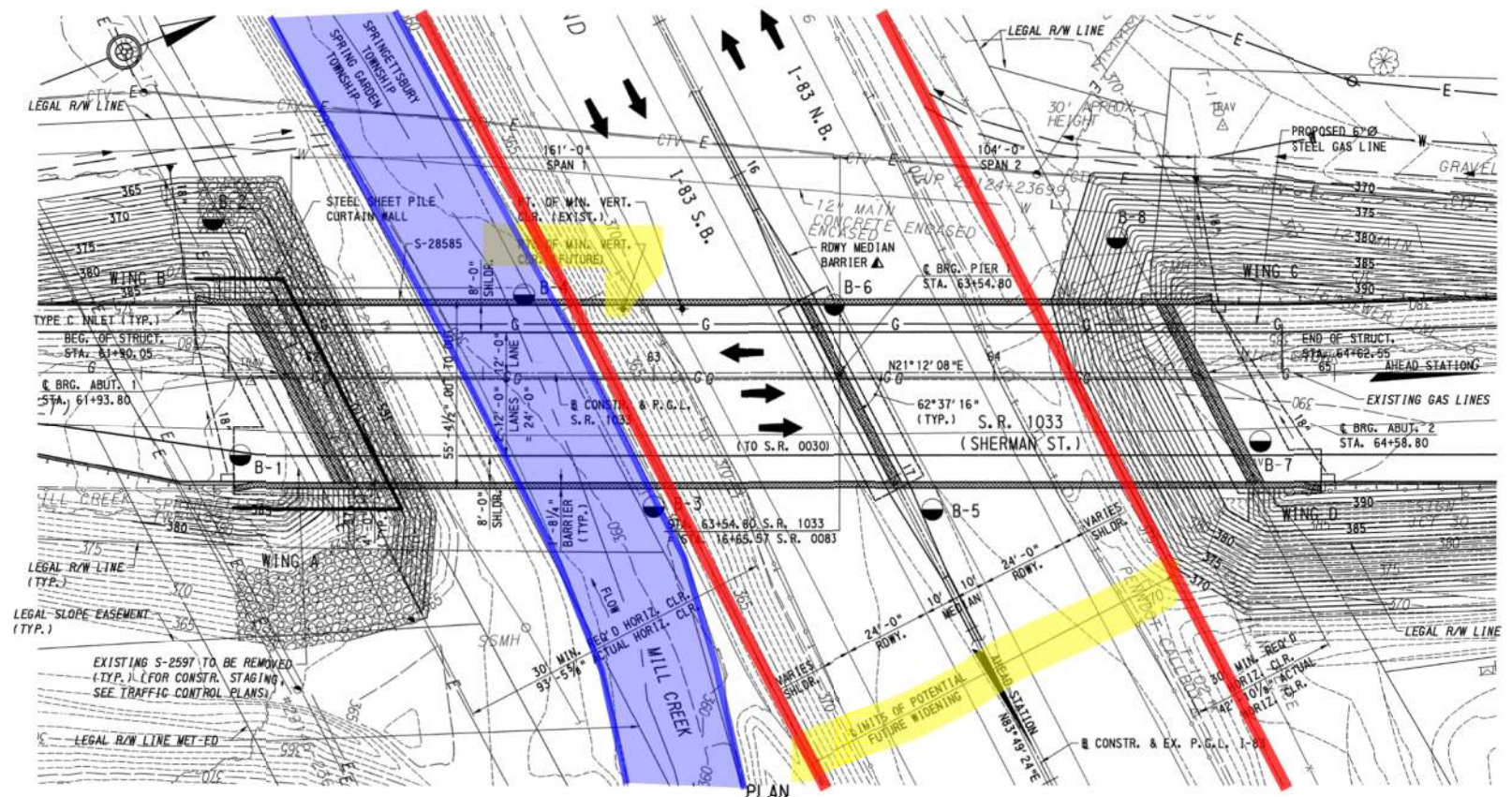




Existing Structure

Constructed In 2012

Designed to Accommodate the Future Widening





Mill Creek





Mill Creek

Flooding reportedly
circa 2006 or 2011
(Hurricane Irene or
Tropical Storm Lee)





Mill Creek

2021

Hurricane Irene



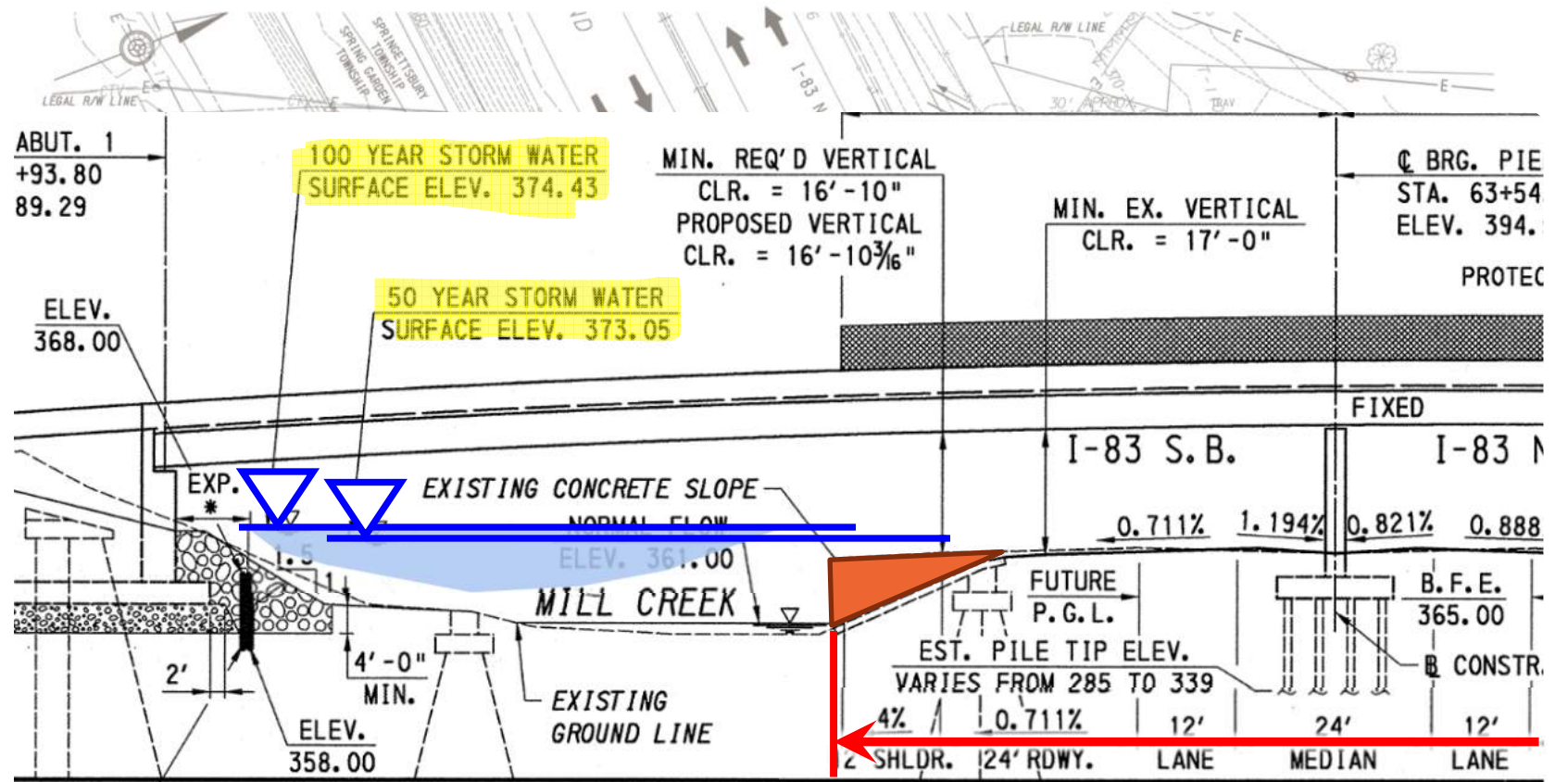


Existing Structure

Constructed in 2012

Designed to accommodate the future widening

Future widening reduced hydraulic opening

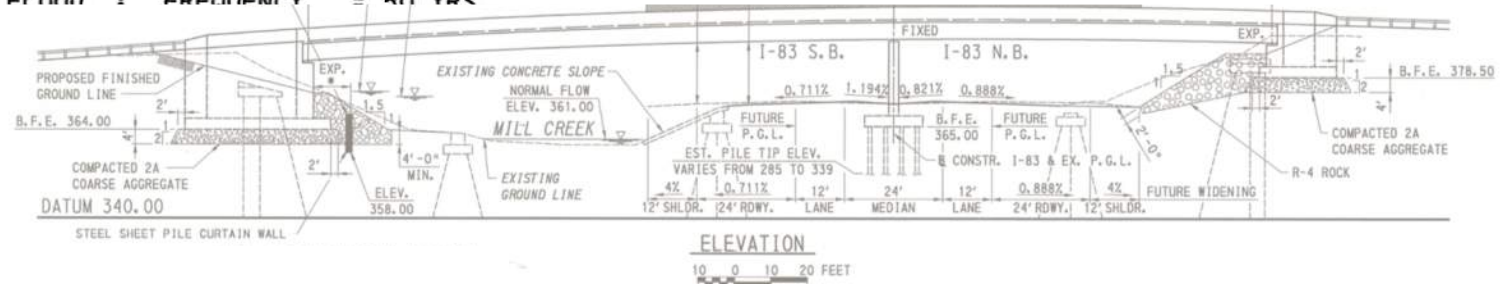
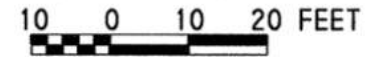


MILL CREEK HYDRAULIC DATA

WATERWAY AREA : 16.84 SQ. MILES

DESIGN FLOOD : FREQUENCY = 50 YRS

ELEVATION





Mill Creek Stream Impacts





Mill Creek Stream Impacts



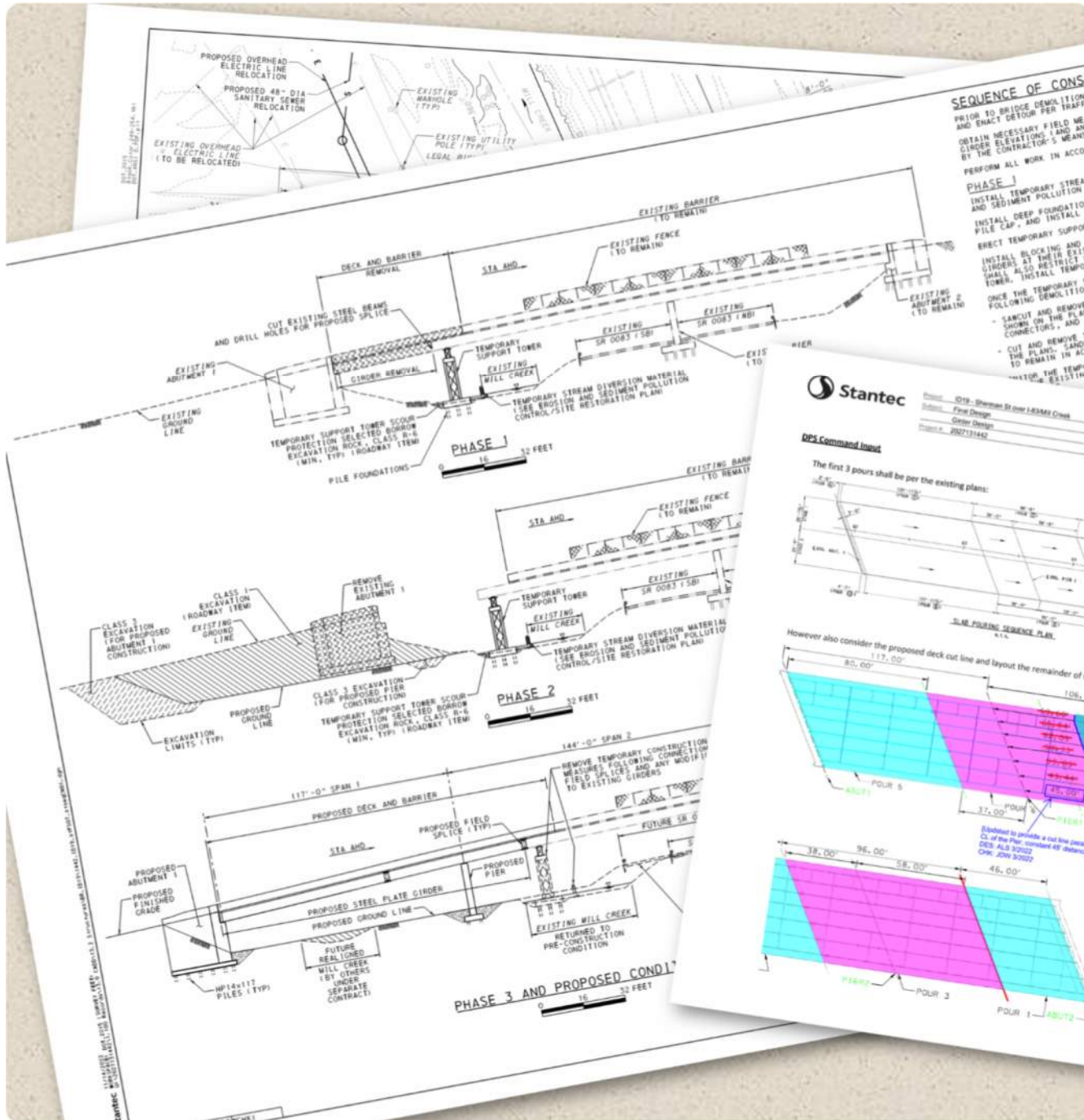


Mill Creek Stream Relocation





Design Challenges and Considerations





New Bridge Length and Splice Locations

Step 1: Determine New Bridge Length

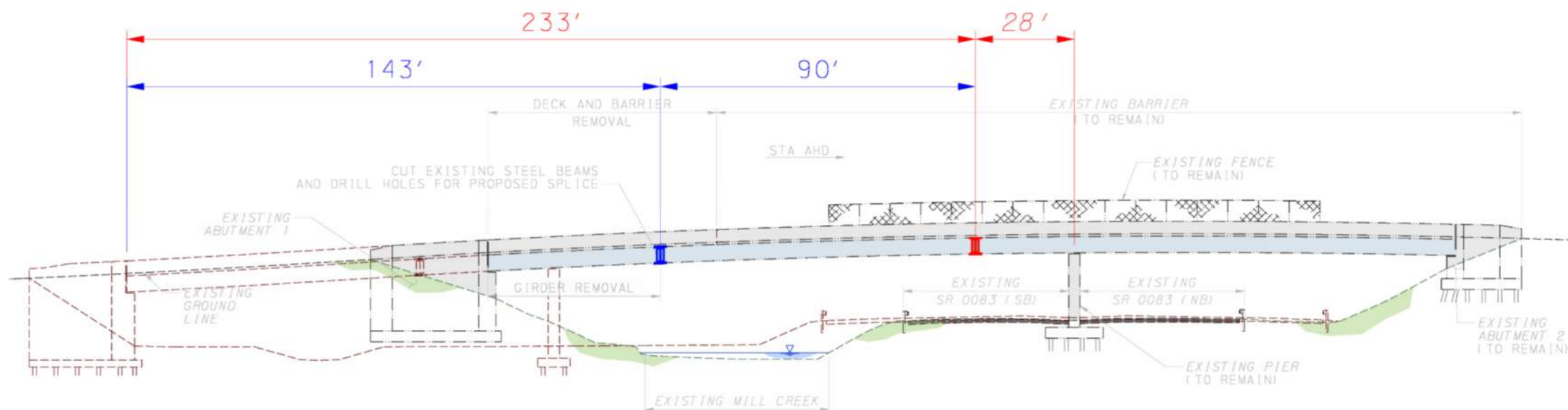
- Profile
- Hydraulic opening

Step 2: Determine Substructure Locations

- Existing substructure location
- Relocated Mill Creek
- Span arrangement

Step 3: Determine Girder Splice Location

- Splice-in at existing splice location?
- Cut existing girders and use new splice location?





Girder Splice

Splice-in at Existing Splice Location

Pros:

- Conventional work – “Common” Approach
- No temporary support required

Cons:

- Removes nearly half of the existing superstructure
- MPT on I-83 required during removal and erection operations – Added Cost

SELECTED!

Cut Existing Girders and Use New Splice Location

Pros:

- No MPT needed on I-83
- Retains 90 feet of the existing superstructure

Cons:

- Requires specialty work to cut and finish the beam
- Requires a temporary support during construction

Estimated cost savings approx. **\$300k**





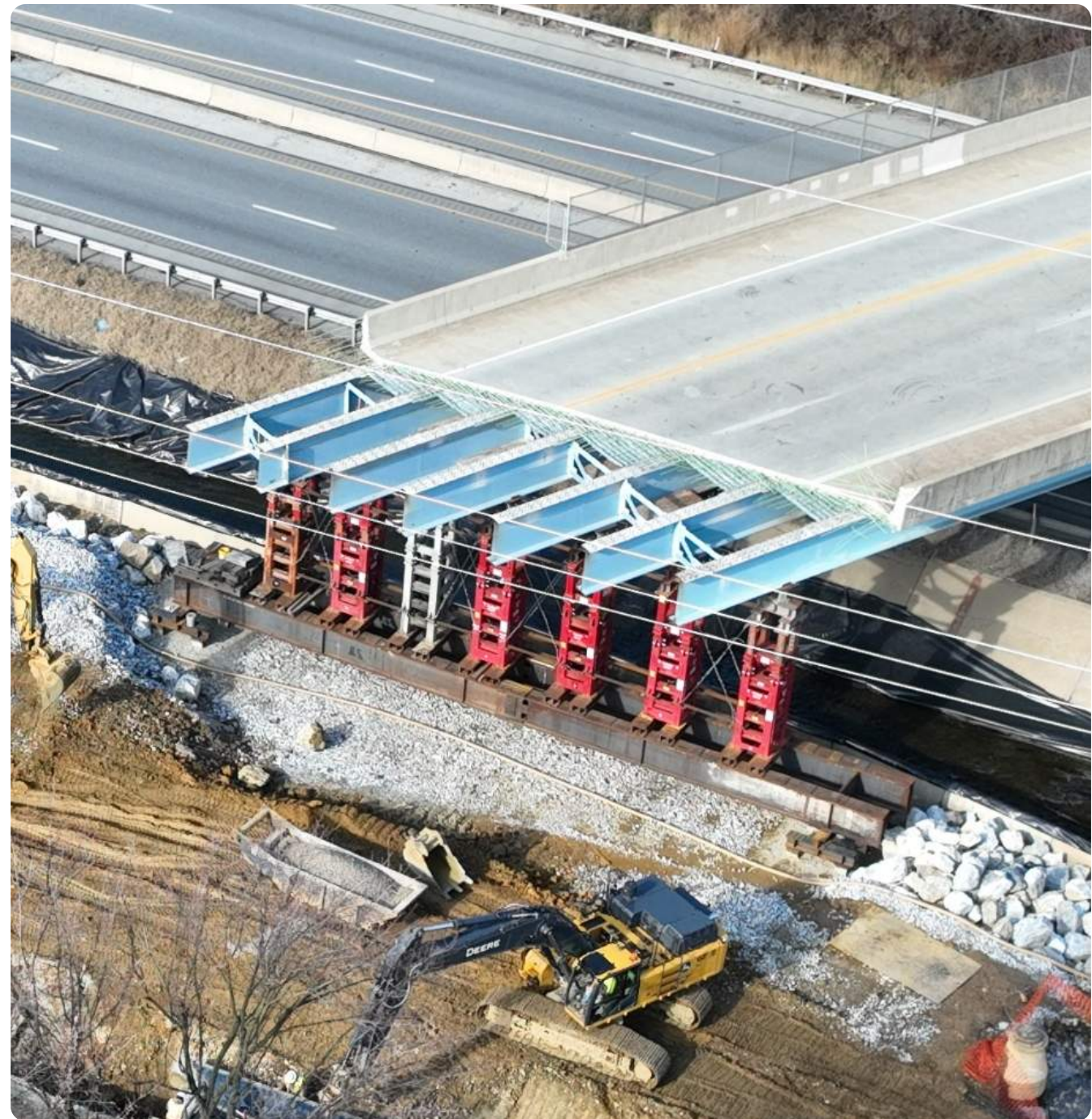
Temporary Support

Scour Considerations

- Provisions for temporary bridges during a flood event, but not temporary supports
- Deep foundations were required

Temporary Support Deflections

- Monitor for movement
- Jacking provisions for fine adjustments





Construction & Challenges





Ground Water and Sanitary Sewer Challenges

Eberts Lane Water Treatment

- Anticipated flow rate was 100 GPM
- Actual flow rate encountered **700 GPM**

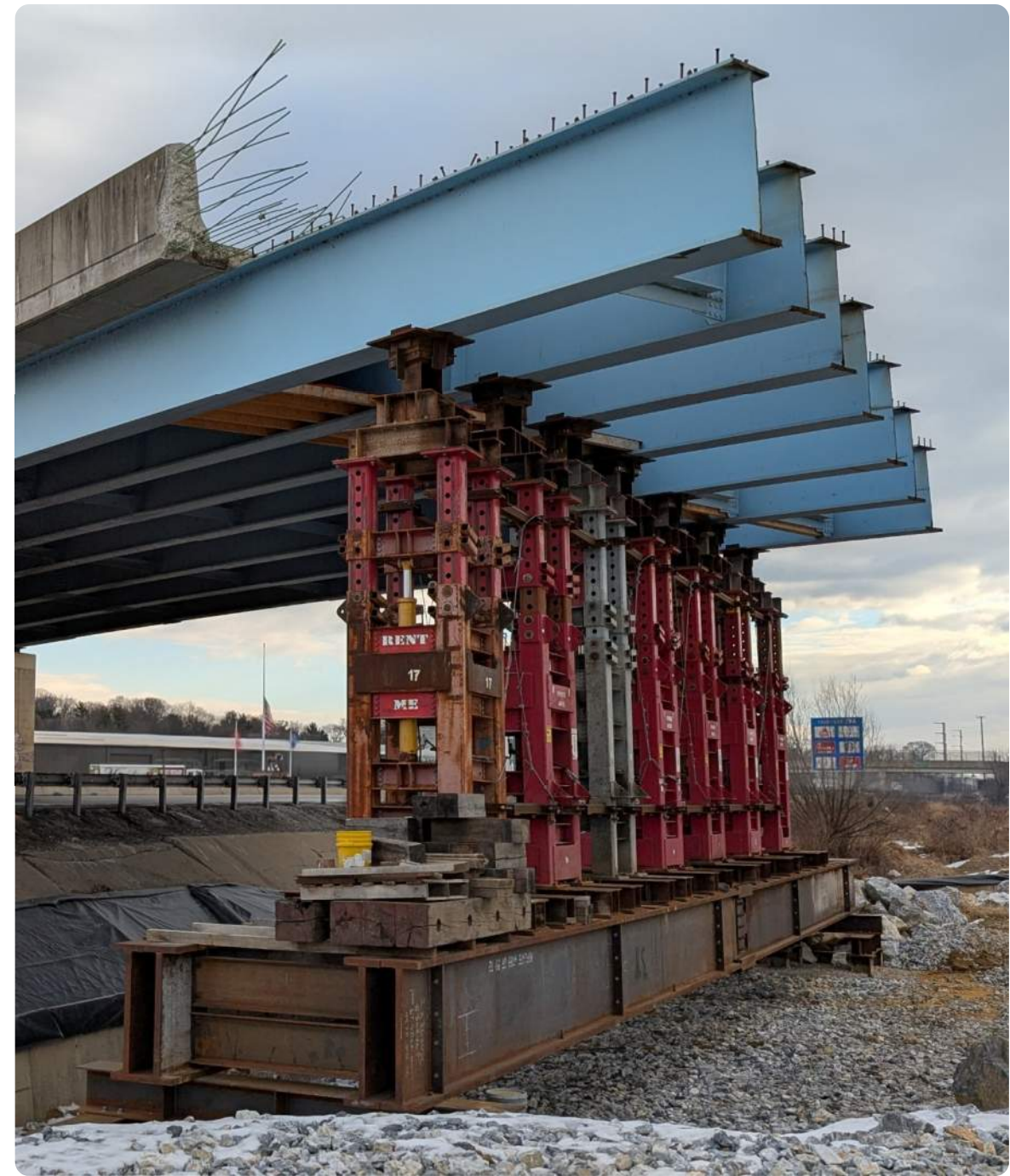
Solutions at Eberts – Applied at Sherman

- Dog Box Inlets for bypass pumping
- Well Points to assist in filtering sediment
- “Agility Concept” between the Department and Springettsbury Township
- Larger Manholes and live switch between the existing Sanitary Sewer System and the relocated system















Foundations

KARST





Existing Structure Foundation Design

Existing plans showed driven piles.

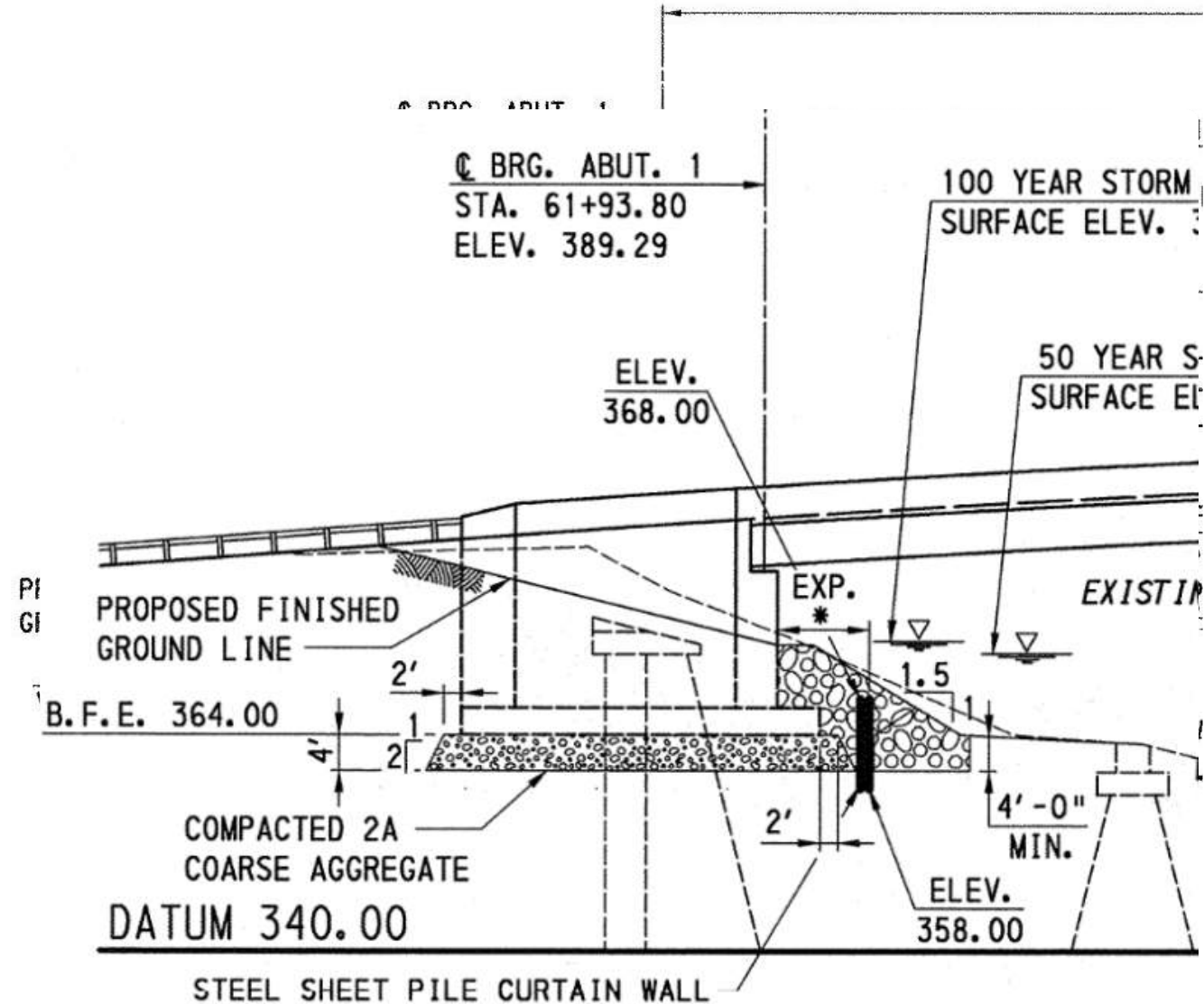
At some point during construction, foundations were converted to spread footings, reportedly due to driving issues.

No documentation of this change was available.

Details of reasons and issues were murky.

Project was a Design-Build (sort of).

Contacted EOR for the Existing bridge.



MAGNITUDE = 1,590 CFS
WATER ELEV. = 373.05
VELOCITY = 11.34 FPS

FREQUENCY = 100 YRS.
MAGNITUDE = 10,065 CFS



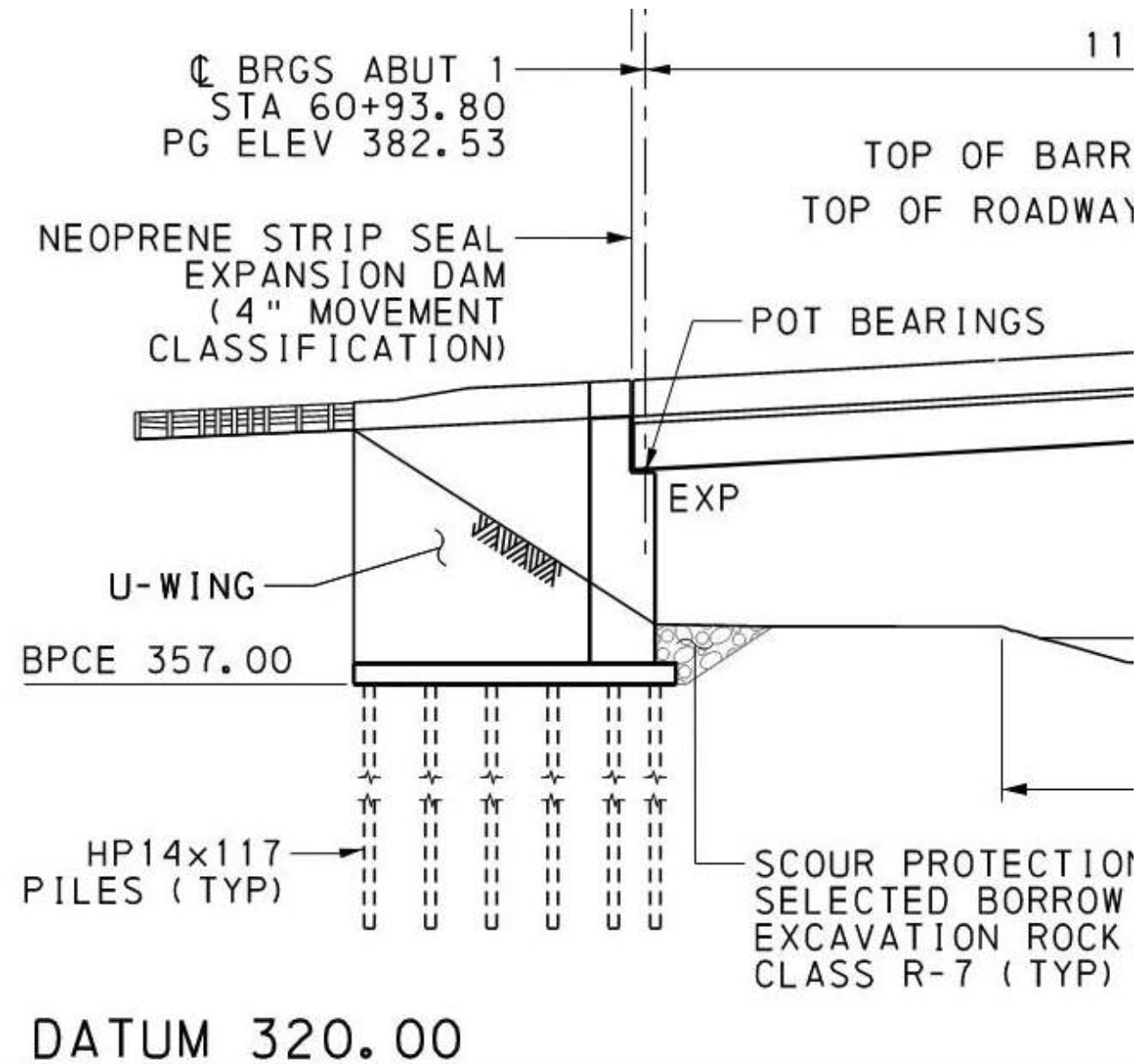




Proposed Foundations

Predrilled Piles with Grouted Tips

- Vertical piles
- Predrilled into 5 ft of rock with 5 ft of competent rock below
- Grouted tips provided lateral resistance
- Air-track drilled to determine depth before drilling





Foundation Options

Predrilling = \$\$\$\$

- Not feasible given the required depth to rock

Use Spread Footings

(like existing bridge)

- Settlement and movement concerns

Drive Piles using a PDA

- Could be costly given depth to rock

Final Decision: Drive Piles using a PDA

- Same pile size and quantity
- Minor design update to account for pile losses
- Provides least risk in the long-term
- Battered piles are not recommended – alternate solutions for lateral resistance

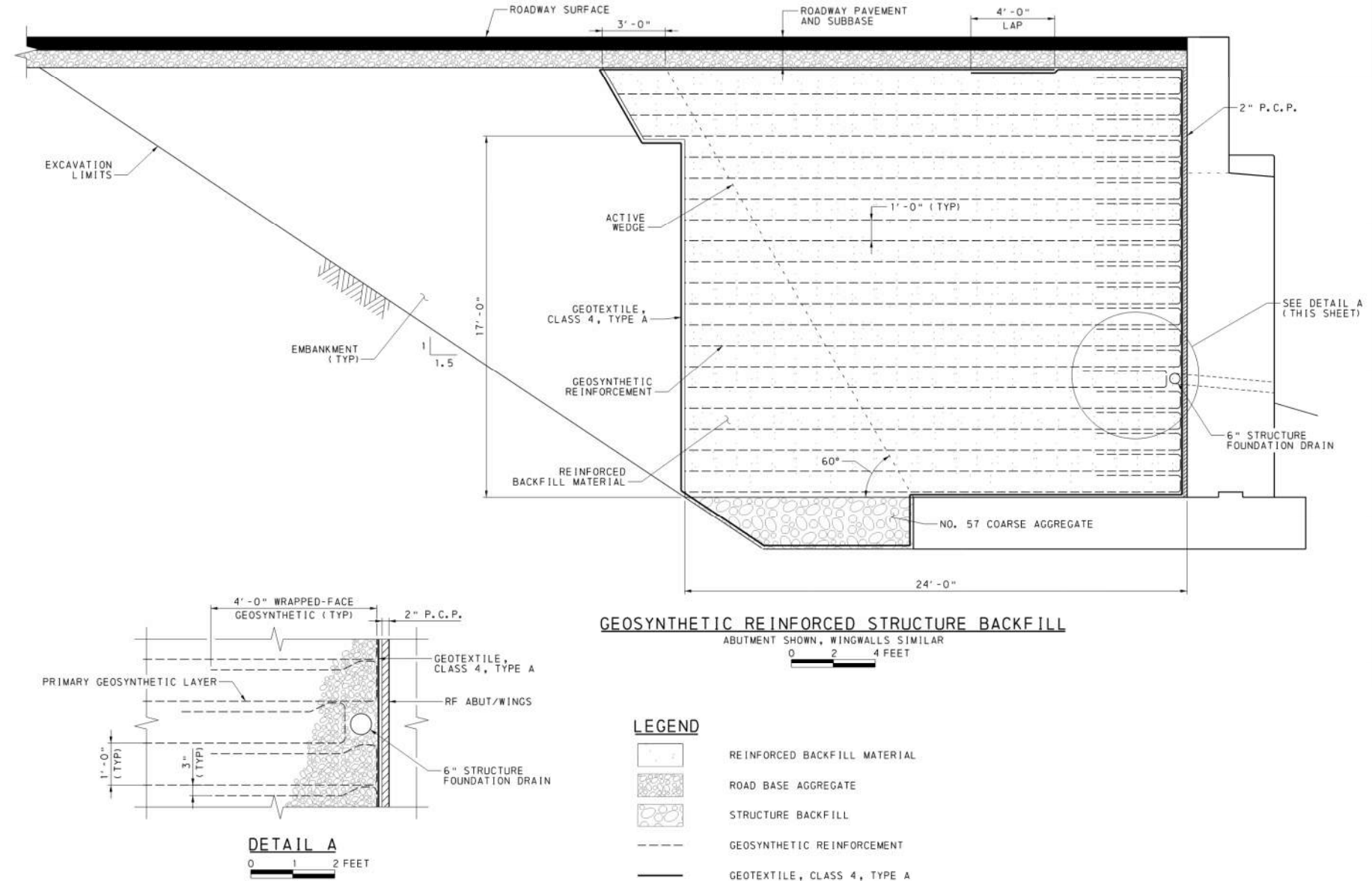




GRS Backfill

Elimination of predrilling reduces lateral capacity of vertical piles

GRS backfill added to reduce lateral loading on the piles





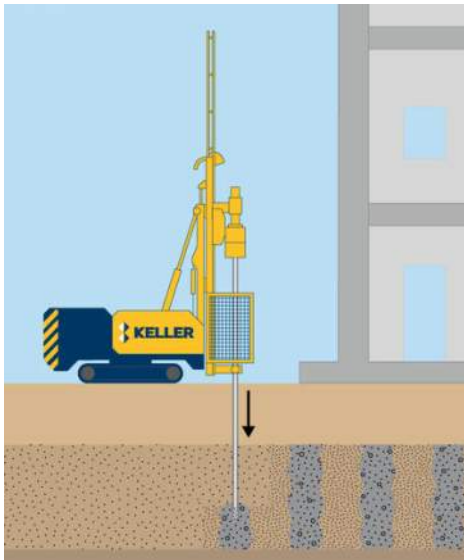




Sinkhole Remediation Options

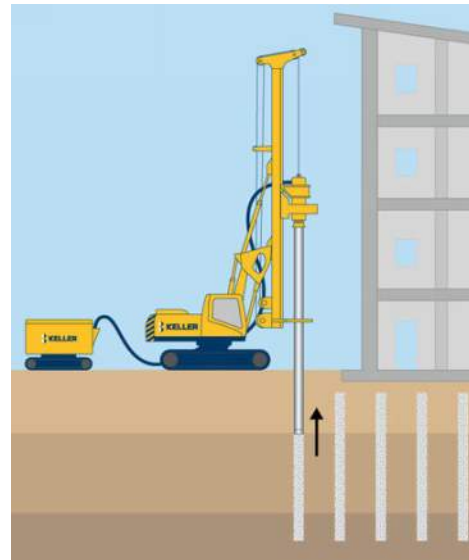
Low Mobility (Compaction) Grouting

- Lowest Risk/Highest Cost
- Est. \$900k



Rigid Inclusions/ Aggregate Column

- Highest Risk/Lowest Cost
- Est. \$300k



Combo of Low Mobility Grouting and Rigid Inclusions

- Medium Risk/Medium Cost
- Est. \$550k

Also looked into other options, such as Dynamic Compaction.

Sinkhole remediation anticipated to be used in combination with spread footings and sheet pile containment





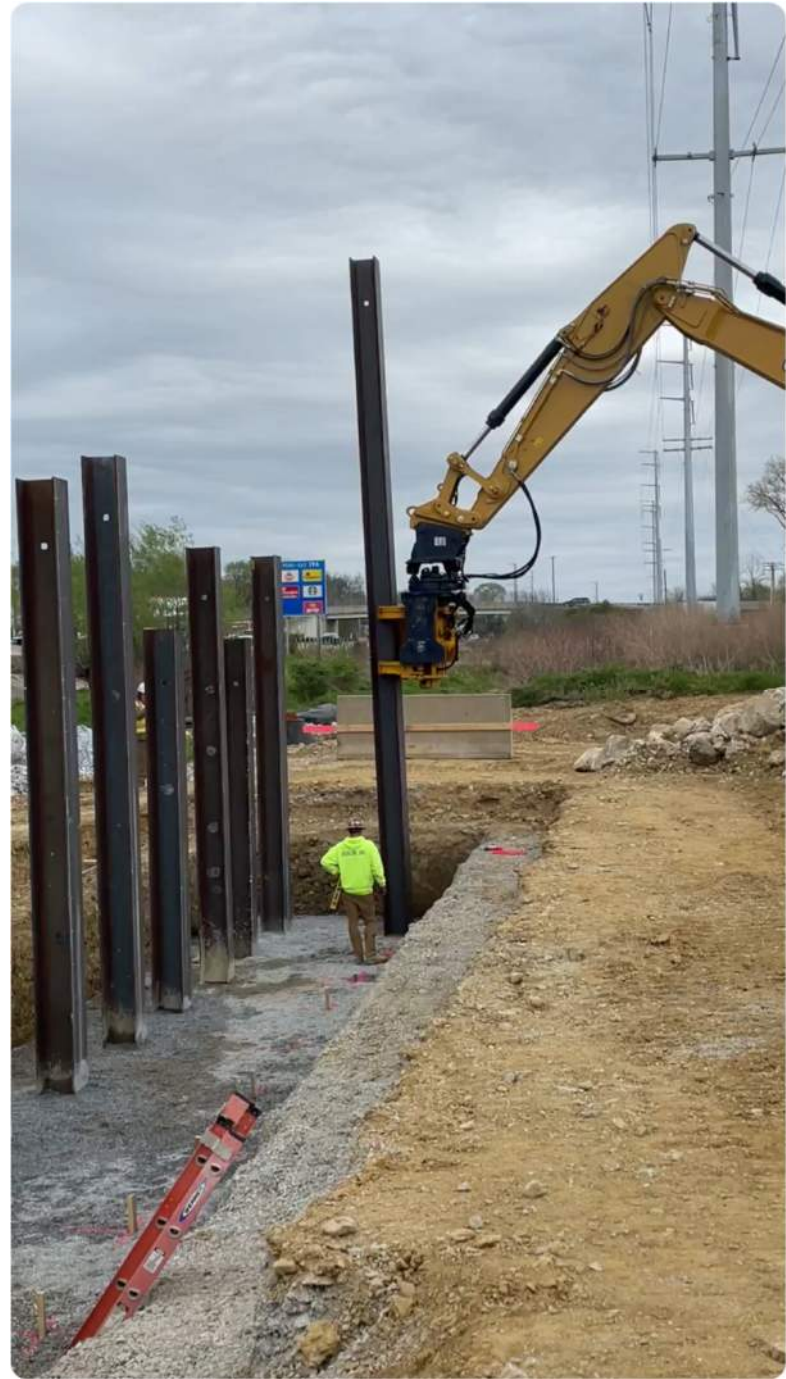
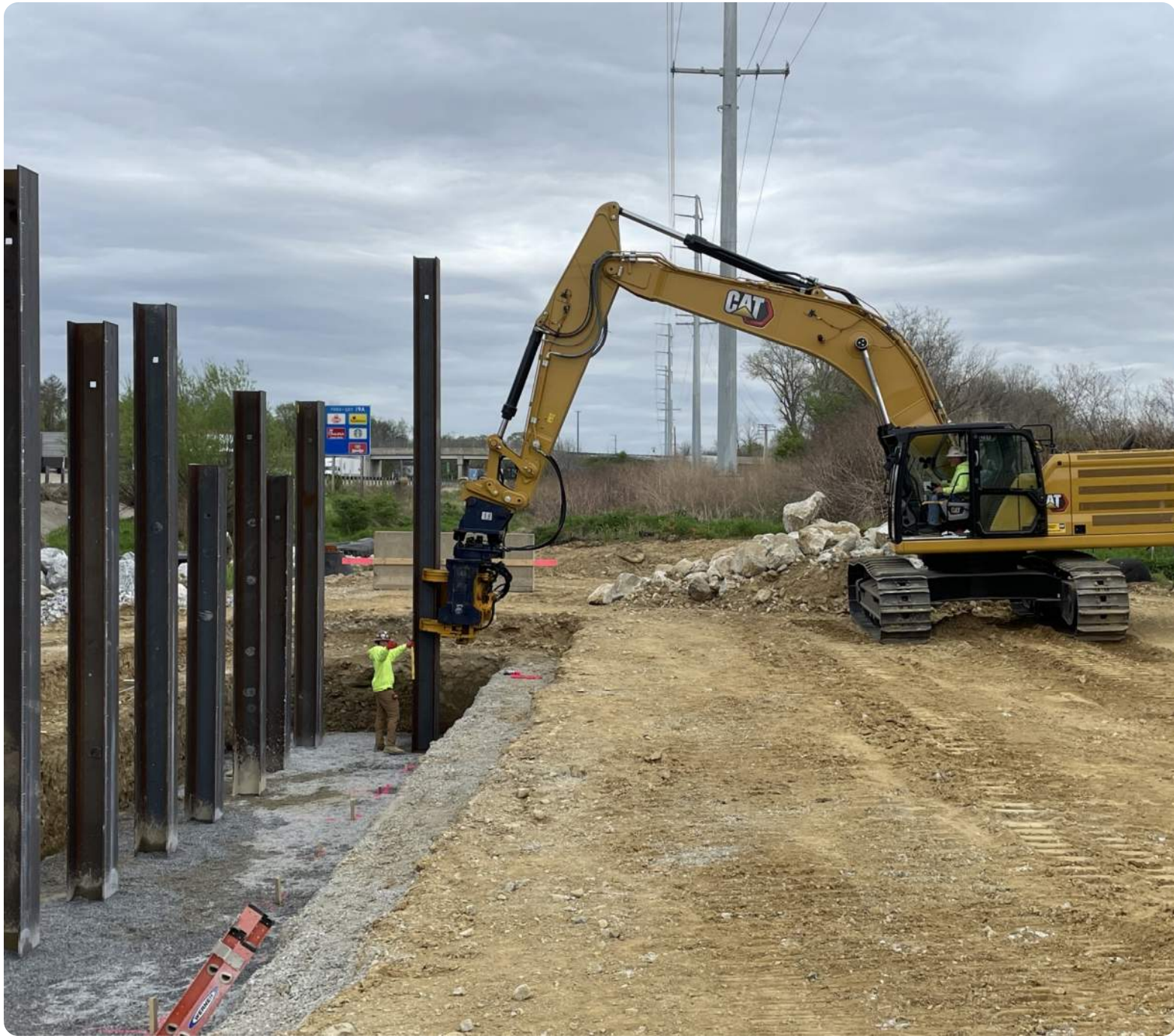
Final Foundation Decision

Stick with driven piles using a PDA to confirm capacity

***“ A Driven Pile is
a Proven Pile ”***

















Conclusions





Construction Costs

Original Bid Price: **\$ 10.1 M**

- Eberts Lane Bridge: **\$ 2.7M**
- Sherman Street Bridge: **\$ 4.4M**

Final Contract Amount: **\$ 11.3 M**

- Added I-83 Repairs: **\$ 760k**
- Eberts Water Treatment: **\$ 430k**
- Sherman Foundations: **\$ 140k**
 - *Additional Work:* **\$ 785k**
 - *Deduct as-designed items:* **(\$ 645k)**
- Misc Adjustments: **(\$ 120k)**

Schedule:

12/31/2024 → 7/19/2025 → **10/13/2025**





Conclusions & Takeaways

- **Use all available resources!**
- **Foundations**
 - Evaluate need for Air-Track Drilling.
 - Additional borings in design?
 - “A driven pile is a proven pile”
- **Cutting vs Splicing**
 - Evaluate Risks vs Cost Benefits





Thank you





Questions?

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